

DEMOLITION NOTES:

1. REMOVE EXISTING TAR AND GRAVEL ROOFING TO DECK. THE DECK IS TO BE CLEAN, FREE AND CLEAR OF DEBRIS AND PREPARED IN A MANNER SATISFACTORY TO ACCEPT THE ADHESIVE SPECIFIED BY THE ROOFING SUPPLIER. CONTRACTOR TO COORDINATE. PROVIDE NEW INSULATION ABOVE DECK AND COVER ROOF WITH A NEW THERMOPLASTIC POLYOLEFIN (TPO) ROOFING SYSTEM OR MODIFIED BITUMEN TORCH DOWN (BTU) ROOFING SYSTEM AS NOTED IN 'NEW ROOF GUIDELINES.
2. EXISTING MOISTURE PREVENTION COUNTERFLASHING TO BE REMOVED AT ALL LOCATIONS WHERE NEW ROOFING SYSTEM WILL BE INSTALLED. REMOVE BY DRILLING OUT EXISTING ANCHORS OR BY OTHERS MEANS THAT WILL NOT CAUSE DAMAGE TO EXISTING STUCCO OR MASONRY WALL. CONTRACTOR TO COORDINATE AS REQUIRED. ANY DAMAGE REPAIR EXPENSE IS THE RESPONSIBILITY OF THE ROOFING CONTRACTOR. EXISTING MOISTURE PREVENTION FLASHING SYSTEM CANNOT BE REUSED. SEE SHEET 103 FOR ADDITIONAL INFORMATION.
3. FOR SPECIFIED DEMOLITION AREAS AND ITEMS, REMOVE AND LEGALLY DISPOSE OF ALL DEMOLITION ITEMS AND DEBRIS WHICH INCLUDE BUT IS NOT LIMITED TO CONCRETE, ASPHALT, CINDER BLOCK, ETC. DURING DEMOLITION, MEASURES SHALL BE TAKEN AS TO ENSURE THAT UTILITIES CONNECTING TO NON-DEMOLITIONED AREAS ARE DISCONNECTED APPROPRIATELY AS TO PREVENT DAMAGE OR LOSS OF UTILITY USE TO NON-DEMOLITIONED AREAS.
4. ASBESTOS TESTING HAS BEEN PERFORMED AND DETERMINED TO BE FREE FROM ANY ASBESTOS. REPORT IS AVAILABLE UPON REQUEST.
5. EXISTING STRUCTURES, MATERIALS, LANDSCAPING, ETC. THAT ARE TO REMAIN THAT ARE DAMAGED IN THE PROCESS OF WORK, ARE THE CONTRACTOR'S RESPONSIBILITY TO REPAIR OR REPLACE IN KIND. THE ENGINEER SHALL BE THE SOLE JUDGE ON THE ACCEPTABILITY OF THE REPAIR OR REPLACEMENT.
6. THE INTENT IS NOT TO DAMAGE THE EXISTING STUCCO. IF DAMAGE OCCURS DURING RE-ROOF, THE ROOFING CONTRACTOR IS TO REPAIR AT OWN EXPENSE.
7. ALL NON-SALVAGE ITEMS SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
8. SECURITY AND SAFETY ARE THE CONTRACTOR'S SOLE RESPONSIBILITY.
9. ALL UTILITIES (WATER, SEWER, TELEPHONE, DRAINS, GAS, ETC.) ARE TO REMAIN IN PLACE UNLESS NOTED OTHERWISE. ALL UTILITIES ARE TO BE PROTECTED FROM DAMAGES.
10. THE ROOFING CONTRACTOR IS TO BRING UP ANY ISSUES ENCOUNTERED BY CONTACTING CITY OF LOVINGTON OR LAMB ENGINEERING & DESIGN AS REQUIRED.

TOTAL EXISTING ROOF AREA:

7,925 + 4,283 + 1,421 ≈ 13,629 TOTAL AREA.
AREA DOES NOT ACCOUNT FOR PARAPET COVERAGE
REQUIRED. CONTRACTOR TO ACCOUNT FOR PARAPET
COVERAGE REQUIRED.

GENERAL EXISTING ROOF NOTES:

- A. NO AREAS OF PONDING HAVE BEEN OBSERVED.
- B. ALL OF THE EXISTING TAR AND GRAVEL ROOFING LOCATIONS SHOWN ON EXISTING ROOF PLAN INDICATES AREAS OF WORK TO BE PERFORMED.
- C. ALL EXISTING HVAC ROOFTOP UNIT CURBS ARE MINIMUM 12" ABOVE EXISTING ROOF.
- D. EXISTING FLASHING IS APPROXIMATELY 3-3/4" DEEP TOTAL.

STRUCTURAL REPAIR SPECIFICATIONS & GENERAL STRUCTURAL NOTES (G.S.N.):

LOADS

GRAVITY:

ROOF:
ROOF LIVE LOAD = 20 PSF (REDUCIBLE).
ROOF DEAD LOAD = 12 PSF.
ROOF SNOW LOAD = 7 PSF.
GROUND SNOW LOAD = 10 PSF.

WOOD:

WOOD ROOF DECKING:

NOMINAL 1X DECKING, H.F. #2 MINIMUM. FOR EDGE ATTACHMENT, USE #10 SCREWS OR 10d NAILS AT 6" O.C. MAXIMUM. FOR INTERMEDIATE ATTACHMENT, USE #10 SCREWS OR 10d NAILS AT 12" O.C. MAXIMUM. FASTENERS ARE TO PENETRATE SUPPORTING FRAMING MEMBER MINIMUM 1".

STEEL DECKING:

ALL STEEL DECK SHALL BE MANUFACTURED AND ERRECTED IN ACCORDANCE WITH LATEST EDITION OF SDI OR APPROVED EQUIVALENT. STEEL DECK SHALL BEAR ON SUPPORTS A MINIMUM OF 2 INCHES. ENDS OF SHEETS MUST BE LAPPED A MINIMUM OF 2 INCHES OVER SUPPORTS.

WELDERS EXPERIENCED IN LIGHT GAGE STEEL DECK WORK SHALL PERFORM ALL WELDING. DECK WELDING MAY BE ACHIEVED WITH E60 SERIES NON LOW HYDROGEN RODS OR E70 SERIES LOW HYDROGEN RODS.

SCREWS WHERE INDICATED SHALL BE #12-24 TRAXX OR APPROVED EQUIVALENT.

ROOF DECK:

DECK SHALL BE 1.5" DEEP, 36" WIDE, 22 GAGE STEEL OR THICKER WITH FINISH PER ARCHITECTURAL DRAWINGS, WITH MINIMUM YIELD STRESS OF 38 KSI, WITH MINIMUM S = 0.187 IN3 AND I = 0.175 IN4 PER FOOT OF WIDTH. DECK SHALL BE ERRECTED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AS 2 SPAN MINIMUM AND SHALL BE ATTACHED FOR A MINIMUM DIAPHRAGM SHEAR CAPACITY OF 303 PLF USING THE FOLLOWING MINIMUM ATTACHMENTS:

WELD ATTACHMENT:

WELD DECK TO SUPPORTING MEMBERS WITH (4) 1/2" DIAMETER OR 3/8" X 1" PUDDLE WELDS PER SHEET AT ENDS, END LAPS AND AT INTERMEDIATE SUPPORTS, AND AT 12" O.C. AT PERIMETER BEAMS AND OPENING EDGES RUNNING PARALLEL TO THE DECK. SIDE SEAM ATTACHMENT SHALL BE BUTTON PUNCHES AT 24" O.C. OR TOP SEAM WELDS AT 24" O.C.

SCREW ATTACHMENT:

ATTACH DECK TO SUPPORTING MEMBERS WITH (4) #12 OR #14 SDI RECOGNIZED SCREWS PER SHEET AT ENDS, END LAPS AND AT INTERMEDIATE SUPPORTS AND AT 12 INCHES ON CENTER AT PERIMETER BEAMS AND OPENING EDGES RUNNING PARALLEL TO THE DECK. SIDE SEAM ATTACHMENT SHALL BE TEKS SCREWS AT 12 INCHES ON CENTER. SCREWS FOR DECK-TO-DECK SEAM FASTENING SHALL BE NO.12-14 x 3/4 INCH TEKS 1. SCREWS FOR DECK TO SUPPORT FASTENING SHALL BE #12-14 X 3/4 INCH TEKS 3 FOR 16 GAGE TO 3/16 INCH SUPPORT THICKNESS, #12-24 X 7/8 INCH TEKS 4 FOR 1/8 INCH TO 1/4 INCH SUPPORT THICKNESS AND/OR #12-24 X 1-1/4 INCH TEKS 5 FOR 1/8 INCH TO 1/2 INCH SUPPORT THICKNESS.

EXPANSION AND EPOXY ANCHORS:

ALL EXPANSIVE ANCHORAGE FOR CONCRETE INSTALLATION SHALL BE PER ITW/REDHEAD "TRUBOLT" WEDGE ANCHOR (ICC ESR-2251) OR APPROVED EQUIVALENT. ALL ADHESIVE (EPOXY) ANCHORAGE FOR CONCRETE SHALL BE PER SIMPSON "SET" SYSTEM WITH DUAL SIDE BY SIDE CARTRIDGES (ICC ESR-1771) OR APPROVED EQUIVALENT.

MASONRY:

GENERAL:

HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, MEDIUM WEIGHT, GRADE N, F'M = 1,500 PSI, RUNNING BOND, MORTAR TYPE S, 1,800 PSI. GROUT 2,000 PSI. MECHANICALLY VIBRATE GROUT IMMEDIATELY AFTER POURING AND AGAIN 5 TO 10 MINUTES LATER. PROVIDE CLEANOUTS IF GROUT LIFT EXCEEDS 5'-0" IN BLOCK WALLS. MAXIMUM GROUT LIFT SHALL BE 6'-0". WHEN APPROVED BY THE STRUCTURAL ENGINEER AND BUILDING OFFICIAL, GROUT LIFTS MAY BE GREATER THAN 6'-0" IF IT CAN BE DEMONSTRATED BY CONTRACTOR THAT THE GROUT SPACES CAN BE PROPERLY FILLED. FILL CELLS SOLIDLY WITH GROUT IN LIFTS AND STOP POURS 11/2" BELOW THE TOP OF A COURSE TO FORM A KEY AT POUR POINTS. UNLESS NOTED OTHERWISE ON THE PLANS, PLACE CONTROL JOINTS IN MASONRY WALLS SUCH THAT NO STRAIGHT RUNS OF WALL EXCEEDS 24'-0". CONTROL JOINTS SHALL NOT OCCUR WITHIN 24" OF CONCENTRATED POINTS OF BEARING OR JAMBS, OR OVER OPENINGS UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID.

